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+0.5000% +1.9500%

UNCLASSIFIED STRUCTURE EXCAVATION

# HYDRAULIC DATA

DESIGN DISCHARGE	440 CFS
FREQUENCY OF DESIGN FLOOD	2 YRS.*
DESIGN HIGH WATER ELEVATION	3205.2
DRAINAGE AREA	6.0 SQ.MI.
BASE DISCHARGE (0100)	2100 CFS
BASE HIGH WATER ELEVATION	3209.33
* MAINTAINS EXISTING LEVEL OF SERVI	CE

OVERTOPPING FLOOD DATA

OVERTOPPING FLOOD DISCHARGE \_\_\_\_\_ 711 CFS FREQUENCY OF OVERTOPPING FLOOD \_\_\_\_\_5± YRS. OVERTOPPING FLOOD ELEVATION \_\_\_\_\_ 3206.76



PROJECT NO. 178P.14.R.115 MACON COUNTY

STATION: 13+09.00 -L-

REPLACES BRIDGE No. 29 SHEET 1 OF 3

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION RALEIGH GENERAL DRAWING

FOR BRIDGE OVER WHITE OAK CREEK

ON SR 1475 BETWEEN SR 1310 AND BALDWIN RD.

		REV	ISION	IS		SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-1
1			3			TOTAL SHEETS
2			4			17





1. FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS. 2. PILES AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 75 TONS PER PILE. 3. DRIVE PILES AT END BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 125 TONS PER PILE. 4. PILES AT END BENT NO. 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 75 TONS PER PILE. 5. DRIVE PILES AT END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 125 TONS PER PILE.

# FOUNDATION NOTES:

		PROJE	CT NO.	<u>17</u> BF	2.14.R	.115
			MAC	NC	CO	UNTY
		STATI	0N: <u>1</u>	3+09	.00 -	<u>L-</u>
		SHEET 2	OF 3			
		DEPA	RTMENT	E OF NORTH CAR OF TRA RALEIGH	OLINA NSPORTA	TION
ſ	DOCUMENT NOT CONSIDERED FINAL	G	ENER	AL DF	RAWIN	١G
	SEAL		FOR WHIT O BETV	BRIDGE E OAK N SR 14 VEEN SR	OVER CREEK 75 1310	
Γ	Indexes and a second se			BALDWI	N RD.	
	Thomas Harris	NO. BY:	REVIS DATE:	NO. BY:	DATE:	SHEET NO. S-2
	F9EBC057AC1A4EF 3/16/2021	1		3		TOTAL SHEETS 17

![](_page_3_Figure_0.jpeg)

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# NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

FOR EROSION CONTROL MEASURES SEE EROSION CONTROL PLANS.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTING OF ONE 20'-6"TIMBER JOIST SPAN WITH TIMBER FLOOR ON TIMBER ABUTMENTS LOCATED AT THE PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED BELOW THE LEGAL LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, THE LOAD LIMIT MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 30 FT.EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER.THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 EVALUATING SCOUR AT BRIDGES", FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

THE CONTRACTOR WILL BE REQUIRED TO CONSTRUCT, MAINTAIN AND AFTERWARDS REMOVE A TEMPORARY STRUCTURE AT STATION 13+30.00 -DET- FOR USE DURING CONSTRUCTION OF THE PROPOSED STRUCTURE. FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY STRUCTURE, SEE SPECIAL PROVISIONS.

THE BRIDGE RAILS ON THE TEMPORARY STRUCTURE SHALL BE DESIGNED FOR THE AASHTO LRFD TEST LEVEL 3 (TL-3) CRASH TEST CRITERIA. FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY STRUCTURE, SEE SPECIAL PROVISIONS.

PROVISIONS.

ΒI	<u>ll</u> OF	F MATE	RIAL								
S A Rete	BRIDGE APPROACH SLABS	REINFORCING STEEL	PILE DRIVING EQUIPMENT SETUP FOR HP 12X53 STEEL PILES	HP 1 Stee	2 X 53 EL PILES	32″ ALASKA RAIL	RIP RAP CLASS II (3'-0" THICK)	GEOTEXTILE FABRIC FOR DRAINAGE	ELASTOMERIC BEARINGS	3'-C PRE CON COR SLA	)'' X STRI CRE ED BS
YDS.	LUMP SUM	LBS.	EA.	NO.	LIN.FT.	LIN.FT.	TON	SO.YDS.	LUMP SUM	NO.	LI
	LUMP SUM					90.0			LUMP SUM	10	
.9		1930	5	5	200		28	32			
.9		1930	5	5	225		28	32			
5.8	LUMP SUM	3860	10	10	425	90.0	56	64	LUMP SUM	10	

![](_page_3_Picture_19.jpeg)

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL

1'-6''						
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	UNLESS ALL SIGNATURES COMPLETED		FOR WHI C BET AND	BRIDGE TE OAK ON SR 14 WEEN SF BALDW	OVER CREEK 475 R 1310 IN RD.	
E STREET	MGINEEP. OF		DEVIC			SHEET NO
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										MOMENT					SHEAR						MOMENT		
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	L I VELOAD F AC TORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	L I VEL OAD F AC T ORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)
		HL-93(Inv)	N/A	$\langle 1 \rangle$	1.23	-	1.75	0.29	1.31	А	EX	22.00	0.60	2.18	А	I	13.00	0.80	0.29	1.23	А	EX	22.00
DESIGN		HL-93(0pr)	N/A		1.69	_	1.35	0.29	1.69	А	EX	22.00	0.60	2.94	А	I	8.50	NZA	-	-	-		-
		HS-20(Inv)	36.000	2	1.51	54.360	1.75	0.29	1.60	А	EX	22.00	0.60	2.58	А	I	8.50	0.80	0.29	1.51	А	EX	22.00
RAIING		HS-20(0pr)	36.000		2.08	74.880	1.35	0.29	2.08	А	EX	22.00	0.60	3.49	А	I	8.50	N/A	-	-	-		-
		SNSH	13.500		2.94	39.690	1.40	0.29	3.91	А	EX	22.00	0.60	7.54	А	I	8.50	0.80	0.29	2.94	А	EX	22.00
		SNGARBS2	20.000		2.38	47.600	1.40	0.29	3.16	А	EX	22.00	0.60	5.58	А	I	8.50	0.80	0.29	2.38	А	EX	22.00
		SNAGRIS2	22.000		2.34	51.480	1.40	0.29	3.08	А	EX	26.50	0.60	5.26	А	I	8.50	0.80	0.29	2.34	А	EX	22.00
		SNCOTTS3	27.250		1.47	40.058	1.40	0.29	1.95	А	EX	22.00	0.60	3.59	А	I	8.50	0.80	0.29	1.47	А	EX	22.00
	S <	SNAGGRS4	37.925		1.30	49.303	1.40	0.29	1.72	А	EX	22.00	0.60	3.11	А	I	8.50	0.80	0.29	1.30	А	EX	22.00
		SNS5A	35.550		1.26	44.793	1.40	0.29	1.68	А	EX	22.00	0.60	3.28	А	I	8.50	0.80	0.29	1.26	А	EX	22.00
		SNS6A	39.950		1.19	47.541	1.40	0.29	1.58	А	EX	22.00	0.60	3.05	А	I	8.50	0.80	0.29	1.19	А	EX	22.00
LEGAL		SNS7B	42.000	$\langle 3 \rangle$	1.13	47.460	1.40	0.29	1.51	А	EX	22.00	0.60	3.13	А	I	8.50	0.80	0.29	1.13	А	EX	22.00
LOAD		TNAGRIT3	33.000		1.46	48.180	1.40	0.29	1.94	А	EX	22.00	0.60	3.61	А	I	8.50	0.80	0.29	1.46	А	EX	22.00
RATING		TNT4A	33.075		1.48	48.951	1.40	0.29	1.96	А	EX	22.00	0.60	3.43	Α	I	8.50	0.80	0.29	1.48	А	EX	22.00
		TNT6A	41.600		1.24	51.584	1.40	0.29	1.65	А	EX	22.00	0.60	3.36	А	I	8.50	0.80	0.29	1.24	А	EX	22.00
	ST	TNT7A	42.000		1.26	52.920	1.40	0.29	1.68	А	EX	22.00	0.60	3.06	А	I	8.50	0.80	0.29	1.26	А	EX	22.00
	L L	TNT7B	42.000		1.31	55.020	1.40	0.29	1.75	А	EX	22.00	0.60	2.94	Α	I	8.50	0.80	0.29	1.31	А	EX	22.00
		TNAGRIT4	43.000		1.25	53.750	1.40	0.29	1.66	А	EX	22.00	0.60	2.80	А	I	8.50	0.80	0.29	1.25	Α	EX	22.00
		TNAGT5A	45.000		1.16	52.200	1.40	0.29	1.55	А	EX	22.00	0.60	2.91	Α	I	8.50	0.80	0.29	1.16	Α	EX	22.00
		TNAGT5B	45.000		1.14	51.300	1.40	0.29	1.51	А	EX	22.00	0.60	2.62	Α	I	35.5	0.80	0.29	1.14	Α	EX	22.00

![](_page_4_Figure_1.jpeg)

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<b>38.</b> (			
179	DESIGNED BY:	J. WHEATLEY	DATE : FEB 2021
00	DRAWN BY:	M. HOGAN	DATE : FEB 2021
72 12	CHECKED BY:	T.KIRSCHBAUM	DATE : FEB 2021
2/12 J:\L	DESIGN ENGINEER OF RECORD:	T.HARRIS	DATE : <u>FEB 2021</u>

![](_page_4_Figure_3.jpeg)

LRFR SUMMARY

LOAD FACTORS:

DESIGN	LIMIT STATE	$\gamma_{\text{DC}}$	$\gamma_{D\mathbf{W}}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MBER

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COMMENT

115

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

	EX	22.00								
	EX	22.00		COMMENT	S:					
	EX	22.00		1.						
	EX	22.00		2.						
	EX	22.00		3.						
	EX	22.00		4.						
	EX	22.00								
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	EX	22.00							TTUO	
	EX	22.00			<u>(</u> #) CO	NIRULL	ING LO	JAD RA	IING	
	EX	22.00			1 DESI	GN LOAD	RATING (F	IL-93)		
	EX	22.00			2 DESI	GN LOAD	RATING (H	15-20)		
	EX	22.00						لد لا		
	EX	22.00								
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•				5/10/2021		2		<b>4</b>		

![](_page_5_Figure_0.jpeg)

![](_page_6_Figure_0.jpeg)

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![](_page_7_Figure_0.jpeg)

CONCRETE	RELE	ASE	STRENGTH
UNIT			PSI
45' UNITS			4000

GRADE 270 S	TRANDS
	0.6″ØL.R.
AREA (SQUARE INCHES)	0.217
ULTIMATE STRENGTH (LBS.PER STRAND)	58,600
APPLIED PRESTRESS (LBS.PER STRAND)	43,950

DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0" × 1'-6"
45' CORED SLAB UNIT (EXT UNITS)	0.6″ØL.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	1 <sup>3</sup> ∕16″ <b>↓</b>
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	5⁄16″ ↓
FINAL CAMBER	<sup>13</sup> /16″
** INCLUDES FUTURE WEARING SURF	FACE
DEAD LOAD DEFLECTION AN	ND CAMBER
DEAD LOAD DEFLECTION AN	<b>ND CAMBER</b> 3'-0" × 1'-6"
DEAD LOAD DEFLECTION AN 45' CORED SLAB UNIT (INT UNITS)	ND CAMBER 3'-0" × 1'-6" 0.6" Ø L.R. STRAND
DEAD LOAD DEFLECTION AN 45' CORED SLAB UNIT (INT UNITS) CAMBER (SLAB ALONE IN PLACE )	ND CAMBER 3'-0"× 1'-6" 0.6" Ø L.R. STRAND 15%6″ ↓
DEAD LOAD DEFLECTION AN 45' CORED SLAB UNIT (INT UNITS) CAMBER (SLAB ALONE IN PLACE) DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	ND CAMBER 3'-0" × 1'-6" 0.6" Ø L.R. STRAND 15%6" ↓ 5%6" ↓

\*\* INCLUDES FUTURE WEARING SURFACE

98.012			
T200179	DESIGNED BY: DRAWN BY: CHECKED BY:	J. WHEATLEY M. HOGAN T. KIRSCHBAUM	DATE : FEB 2021 DATE : FEB 2021 DATE : FEB 2021
J:\L	DESIGN ENGINEER OF RECORD:	T.HARRIS	DATE : FEB 2021

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# BAR TYPES

![](_page_7_Figure_8.jpeg)

	BILL OF MATERIAL FOR ONE 45' CORED SLAB UNIT									
	EXTERIOR UNIT INTERIOR UNIT									
	BAR	NUMBER	SIZE	TYPE	LENGTH	WEIG	нт	LENGTH	WEIGHT	
	B1	4	#4	STR	23'-3"	62		23'-3"	62	
	S1	8	<b>#</b> 5	2	3'-9″	31		3′-9″	31	
	S2	128	<b>#</b> 5	2	4'-10"	645	5	4'-10"	645	
	<b>*</b> S3	16	<b>#</b> 4	STR	3'-0"	32				
	<b>*</b> S4	38	#4	1	5′-6″	139				
	REINFO	ORCING	STEEL	LBS	S <b>.</b>	738	3		738	
	* EP0>	Y COATE	ED							
	REIN	FORCINO	<u>S STEEL</u>	LB	<u>S.</u>	171				
	6000	P.S.I.CO	NCRETE	CU. YDS	) .	6.8	3		6.0	
						. –				
	0.6" Ø L.R. STRANDS			No	o. 15			15		
4 '	TERIAL FOR 2 CONCRETE CURBS AND 4 END POSTS									
Ŷ٨	IR OF E	XTERIO	R UNITS	T0	TAL NO.	SIZE	TYPE	LENGTH	WEIGHT	

[	BILL OF MATERIAL FOR 2 CONC	RETE CURBS	S AND	4 EN	D POST	S
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT
	45' UNIT					
<b>*</b> B12	8	8	#4	STR	22'-1"	118
<b>₩</b> E1	40	40	<b>#</b> 7	STR	2'-8"	218
<b>米</b> F1	32	32	<b>#</b> 6	STR	3′-5″	164
<b>∗</b> EPOX	Y COATED REINFORCING STEEL	8		LBS.		500
CLASS	AA CONCRETE			CU.YDS.		7.3
TOTAL				LN.FT.		90.00

CORED SLABS REQUIRED							
	NUMBER	LENGTH	TOTAL LENGTH				
45' UNIT							
EXTERIOR C.S.	2	45'-0"	90'-0"				
INTERIOR C.S.	8	45'-0″	360′-0″				
TOTAL	10	45'-0"	450'-0"				

![](_page_7_Picture_12.jpeg)

# NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.
ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.
RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.
THE $2^{1/2}$ "Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.
THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER.SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.
WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.
ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.
PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.
APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.
FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.
THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE ``CONCRETE RELEASE STRENGTH'' TABLE.
FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.
THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.
THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.
THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.
THE COST OF THE CLASS AA CONCRETE AND REINFORCING STEEL FOR THE CONCRETE CURB AND END POSTS ARE INCLUDED IN THE PRICE BID FOR LINEAR FEET OF 32"ALASKA RAIL.
ANCHOR BOLTS, NUTS WASHERS AND PLATES SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.
ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A449. NUTS SHALL MEET THE REQUIREMENTS OF AASHTO M291-DH OR AASHTO M292-2H. WASHERS SHALL MEET THE REQUIREMENTS OF AASHTO M293. SHOP DRAWINGS ARE NOTE REQUIRED FOR ANCHOR BOLTS, NUTS, WASHERS AND PLATES. SHOPE INSPECTION IS REQUIRED.
AT ALL FIXED ENDS OF CORED SLAB UNITS, NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FIGNER TIGHT AND THEN BACKED OFF $\frac{1}{2}$ TURN. THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURRED WITH A SHARP POINTED TOOL.
PROJECT NO 178P.14.R.115
STATION. 13+09.00 -L-
SHEET 3 OF 3
STATE OF NORTH CAROLINA
DEPARIMENT OF TRANSPORTATION RALEIGH
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED CAROUNIC CAROUNIC CAROUNIC CAROUNIC CAROUNIC CAROUNIC CONCRETE SEAL SEAL SEAL SIGNATURES COMPLETED
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F9EBC057AC1A4EF 3/16/2021USHEETS3/16/20212417

![](_page_8_Figure_0.jpeg)

MATERIAL AND GALVANIZING ARE TO CONFORM TO THE FOLLOWING SPECIFICATIONS: POST, POST BASES, ANCHOR PLATES AND RAIL SPLICE TUBES: AASHTO M270 GRADE 36 STRUCTURAL STEEL-GALVANIZED TO AASHTO M111.

THE CUT ENDS OF GALVANIZED STEEL RAILING, AFTER GRINDING SMOOTH SHALL BE GIVEN TWO COATS OF ZINC RICH PAINT MEETING THE REQUIREMENTS OF FEDERAL SPECIFICATION MIL-P-26915 USAF TYPE 1, OR OF FEDERAL

RAILS: ASTM A500 GRADE B - GALVANIZED TO AASHTO M111. WELDED RAIL STUDS: ASTM A108-GALVANIZED TO AASHTO M111. HIGH STRENGTH ANCHOR BOLTS SHALL CONFORM TO ASTM F1554 GRADE 105. HEAVY HEX NUTS SHALL CONFORM TO ASTM A563 DH, AND WASHERS TO ASTM F436, TYPE 1. ANCHOR BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED

FOR END OF RAIL TO CLEAR FACE OF CONCRETE END POST DIMENSION. SEE SHEET 2 OF 2. METAL RAIL POSTS SHALL BE SET NORMAL TO CURB GRADE.

TO INSURE FUTURE IDENTIFICATION OF THE FABRICATOR, A PERMANENT IDENTIFYING MARK SHALL BE PLACED ON EACH POST. THE METHOD OF MARKING AND LOCATION SHALL BE SUCH THAT IT DOES NOT DETRACT FROM THE APPEARANCE OF THE POST, BUT REMAINS VISIBLE AFTER RAIL PLACEMENT.

SHIMS SHALL BE USED AS NECESSARY FOR POST ALIGNMENT. GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE CURB AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN CURB EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF CURB SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

THE RAIL SECTIONS SHALL BE ATTACHED TO THE POSTS BY TWO THREADED  $\frac{3}{4}$ "  $\varnothing$  welded studs, plate washers,

FOR 32" ALASKA RAIL, SEE THE STANDARD SPECIFICATIONS.

METAL RAIL LENGTH <u>90</u> LIN.FT.

NOTES

METAL RAIL SHALL BE GALVANIZED STEEL IN ACCORDANCE WITH THE REQUIREMENTS OF THE GENERAL NOTES AND THE FOLLOWING SPECIFICATIONS. ALUMINUM RAIL WILL NOT BE AN OPTION.

GALVANIZED STEEL RAILS

GENERAL NOTES

RAILING SHALL BE CONTINUOUS FROM END POST TO END POST OF BRIDGE. EACH JOINT IN RAIL LENGTH SHALL BE SPLICED AS DETAILED. PANEL LENGTHS OF RAIL SHALL BE ATTACHED TO A MINIMUM OF THREE POSTS.

CERTIFIED MILL REPORTS ARE REQUIRED FOR RAILS AND POSTS. SHOP INSPECTION IS NOT REQUIRED.

CURVED RAIL USAGE: WHERE RAILS ARE TO BE USED ON BRIDGES ON HORIZONTAL AND/OR VERTICAL CURVATURE THE CONTRACTOR MAY, AT HIS OPTION, HAVE THE REQUIRED CURVATURE IN THE RAIL FORMED IN THE SHOP OR IN THE FIELD. IN EITHER EVENT, THE RAIL SHALL CONFORM WITHOUT BUCKLING OR KINKING TO THE REQUIRED CURVATURE IN A UNIFORM MANNER ACCEPTABLE TO THE ENGINEER.

	l	PROJECT NO. 17BP.14.R.115						
		MACON COUNTY						
		STATI	ON: 13	3+09.	.00 -	L-		
<u>5″</u> <u>1½</u>	-	SHEET 1	OF 2					
		DEPA	STATE (	OF NORTH CAR DF TRAN RALEIGH	OLINA NSPORTA	TION		
∽ų plate ON	DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED		ST	ANDAF	RD			
_	SEAL 19299	3	2″ ALA	SKA	RAII	_		
E STREET	THE SWOINEER HARTIN		REVISI	ONS		SHEET NO.		
01 0	Thomas Harris	NO. BY:	DATE: N	0. BY:	DATE:	S-8 Total		
165	F9EBC057AC1A4EF 3/16/2021	2		<u>ه</u> }		SHEETS		

STD. NO. BMR8 (SHT 1)

![](_page_9_Figure_0.jpeg)

![](_page_9_Figure_1.jpeg)

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EACH STRUCTURAL CONCRETE INSER

- A. FERRULE SHALL BE MADE FROM SHALL HAVE A MINIMUM LENG
- B. 1 ¾" ∅ X 15%" BOLT WITH AND WASHER SHALL BE GALVAN MAY BE USED AS AN ALTERNA CONFORM TO OR EXCEED THE SHALL BE APPROVED BY THE E
- C. WIRE STRUT SHOWN IN THE ST SHALL HAVE A MINIMUM TENS A MINIMUM TENSILE STRENGT

EACH METAL RAIL TO END POST C

- A. 1/2" METAL BRACKET PLATE AND AND SHALL BE GALVANIZED AFTER FABRICATION TO AASHTO M111.
- SHALL HAVE N.C. THREADS.

THE CONTRACTOR, AT HIS OPTION, MAY USE AN ADHESIVE BONDING SYSTEM IN LIEU OF THE STRUCTURAL CONCRETE INSERT EMBEDDED IN THE END POST. IF THE ADHESIVE BONDING SYSTEM IS USED, THE  $\frac{3}{4}$ " Ø x  $\frac{15}{8}$ " BOLTS WITH WASHERS SHALL BE REPLACED WITH  $\frac{3}{4}$ " Ø x  $\frac{6}{2}$ " BOLTS AND 2" O.D. WASHERS. ALL SPECIFICATIONS THAT APPLY TO THE  $\frac{3}{4}$ " Ø x  $\frac{15}{8}$ " BOLTS SHALL APPLY TO THE  $\frac{3}{4}$ " Ø x  $\frac{6}{2}$ " BOLTS. FIELD TESTING OF THE ADHESIVE BONDING SYSTEM IS NOT REQUIRED.

![](_page_9_Figure_16.jpeg)

![](_page_9_Figure_17.jpeg)

![](_page_9_Picture_20.jpeg)

NOTES
STRUCTURAL CONCRETE INSERT
RT ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:
A STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND TH OF THREADS OF $1^{1}/_{2}^{\prime\prime}$ .
WASHER.BOLT SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307.BOLT NIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLT AND WASHER TE FOR THE $\frac{3}{4}$ " Ø X 1 $\frac{5}{8}$ " GALVANIZED BOLT AND WASHER.THEY SHALL MECHANICAL REQUIREMENTS OF ASTM A307.THE USE OF THIS ALTERNATE ENGINEER.)
TRUCTURAL CONCRETE INSERT DETAIL IS THE MINIMUM ALLOWABLE SIZE AND ILE STRENGTH OF 100,000 PSI. AS AN OPTION, A $\frac{7}{16}$ " Ø WIRE STRUT WITH H OF 90,000 PSI IS ACCEPTABLE.
NOTES
METAL RAIL TO END POST CONNECTION
CONNECTION SHALL CONSIST OF THE FOLLOWING COMPONENTS:
D 1/4" METAL RAIL INSERT TUBE SHALL CONFORM TO AASHTO M270 GRADE 36

B. 3/4" STRUCTURAL CONCRETE INSERTS SHALL HAVE A WORKING LOAD SHEAR CAPACITY OF 4800 LBS. THE FERRULES SHALL ENGAGE A 34" Ø X 158" BOLT WITH 2" O.D. WASHER IN PLACE. THE 34" Ø X 158" BOLT

THE  $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERTS WITH BOLTS SHALL BE ASSEMBLED IN THE SHOP.

THE COST OF THE  $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT, THE  $\frac{1}{2}$ " BRACKET PLATES, AND THE RAIL INSERT TUBES COMPLETE IN PLACE SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

![](_page_10_Figure_0.jpeg)

<u>Plan of curb</u>

![](_page_10_Figure_2.jpeg)

<u>END VIEW</u>

CURB AND END POST FOR 32" ALASKA RAIL

Масо				
R.115				
<b>98.</b> 012				
T200179	DESIGNED BY: DRAWN BY: CHECKED BY:	J. WHEATLEY M.HOGAN T.KIRSCHBAUM	DATE : DATE : DATE :	FEB 2021 FEB 2021 FEB 2021
J.:L	DESIGN ENGINEER OF RECORD:	T.HARRIS	DATE :	FEB 2021

+

<u>PLAN OF END POST</u>

![](_page_10_Figure_9.jpeg)

<u>ELEVATION</u>

![](_page_10_Picture_11.jpeg)

# NOTES:

FOR REBAR LIST AND BILL OF MATERIAL FOR CURB AND END POST, SEE SHEET S-7.

# PROJECT NO. 178P.14.R.115 MACON \_\_\_\_ COUNTY STATION: 13+09.00 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

END POST DETAILS FOR ALASKA RAIL

		SHEET NO.				
0.	BY:	DATE:	NO.	BY:	DATE:	S-10
] [			3			TOTAL SHEETS
2			4			17

![](_page_10_Picture_18.jpeg)

![](_page_11_Figure_0.jpeg)

![](_page_12_Figure_0.jpeg)

# NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE CONCRETE CURB IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4. FOR WING DETAILS, SEE SHEET 3 OF 4.

### PROJECT NO. 178P.14.R.115 MACON COUNTY STATION: 13+09.00 -L-STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETE SUBSTRUCTURE H CARO END BENT 1 SE AL 19299 TOMAS M. HAR SHEET NO. REVISIONS S-12 NO. BY: DATE: DATE: BY: thomas Harris total sheets 17 -F9EBC057AC1A4EF... 3/16/2021

STD. NO. EB\_30\_90S

![](_page_13_Figure_0.jpeg)

# NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS

STD. NO. EB\_30\_90S

![](_page_14_Figure_0.jpeg)

:

![](_page_14_Figure_4.jpeg)

SE AL 19299

# 3"HIGH B.B. SECTION Y-Y PROJECT NO. 178P.14.R.115 MACON COUNTY STATION: 13+09.00 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

SHEET 3 OF 4

# SUBSTRUCTURE END BENT

WING DETAILS

		REVIS	510	NS		SHEET NO.
<b>).</b>	BY:	DATE:	NO.	BY:	DATE:	S-14
]			3			TOTAL SHEETS
2			4			17
		STD.N	10.	.EB_3	30_90S	

![](_page_15_Figure_0.jpeg)

TYPES	BILL OF MATERIAL					
		FOF	R ON	IE E	ND BE	NT
	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
	B1	8	#9	1	38'-0"	1034
7	B2	16	#4	STR	19'-1"	204
→ нк.  нк.	B3	9	#4	STR	2′-5″	15
(4)						
	D1	4	<b>#</b> 6	STR	1'-6"	9
/						
	H1	24	#4	2	7'-10"	126
	14.4	10		C T D	0/ 11/	07
	KI	12	#4	SIR	2'-11"	23
	<u> </u>	16	#1	7	7'-5"	220
$\left(\begin{array}{c} \overline{5} \end{array}\right)$	51 52	40	<sup></sup> 4 #⊿		7-5	220 97
	52 53	10	 #∠	5	6'-6"	43
	55 54	4	#4	6	4'-5"	12
			· · ·			
1'-8"Ø	V1	48	#4	STR	4'-4"	139
1′-5″						
	REINF	ORCIN	NG STE	EL		
	(FOR (	ONE E	ND BEN	(T)	1	930 LBS.
	CLASS	A CO	ONCRET	E BRE	AKDOWN	
		(FOR (	ONE ENI	D BEN	Γ)	
	POUR	<b>#</b> 1 C	AP,LOW	IER PA	RT	11.2 C.Y.
<u>▼</u>		0	F WINC	S & (	COLLARS	
		# <u>)</u> וו			Ē	16 C V
UNS ARE UUT TO UUT.	FUUR	Ψ2 U W	INGS	ARI U		1.0 0.1.
END BENT No.2						
HP 12 X 53 STEEL PILES						
NO:5 LIN.FT.= 225	TOTAL	CLAS	SS A C	ONCRE	TE	12.8 C.Y.
PILE DRIVING EQUIPMENT						
HP 12 X 53 STEEL PILES						
NO <b>:</b> 5						

![](_page_16_Figure_0.jpeg)

+

![](_page_16_Figure_4.jpeg)

END BENT 2

![](_page_16_Figure_7.jpeg)

Ę	SECT	ION
BERM	RIP	RAPPED

![](_page_16_Picture_9.jpeg)

NOTES : FOR BERM WIDTH DIMENSIONS, SEE GENERAL DRAWING.

ESTIMATED QUANTITIES				
BRIDGE @ STA.13+09.00 -L-	RIP RAP CLASS II (3'-0" THICK)	GEOTEXTILE FOR DRAINAGE		
	TONS	SQUARE YARDS		
END BENT 1	28	32		
END BENT 2	28	32		

		PROJECT NO. <u>17BP.14</u> <u>MACON</u> STATION: <u>13+09.00</u>				<u>2.14.R</u> cc .00 -	<u>.R.115</u> county -L-		
		STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					TION		
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ET	Docusigned by	REVISIONS SHEET NO							
	thomas Harris	NO. BY:	DATE:	NO.	BY:	DATE:	S-16		
	3/16/2021	2		৩ 4			SHEETS		

![](_page_17_Figure_0.jpeg)

\_\_\_\_\_ F OR AND GEOT SPEC SELE ACCC SELE BACK FOR AREA DRAI BE P APPF

	c		BI	[LL O	F MA	TERIA	_
INUIE	J	- Г	APPR	ЭАСН	SLAF	BATE	B #1
BRIDGE APPROACH FILL INCLUDING GEO SELECT MATERIAL BACKFILL. SEF ROAD	DTEXTILE, 4″Ø DRAINAGE PIP WAY PLANS.	₽, ┣	BAR NO.	SIZE	TYPE	LENGTH	WEIGHT
TEXTILE SHALL BE TYPE 1 IN ACCORDAN	CE WITH THE STANDARD	F	* A1 13	<b>#</b> 4	STR	28'-10"	250
CIFICATIONS SECTION 1056.		┣	AZ 13	# <u>4</u>	51K	∠8 <sup>-</sup> -10" •	250
ECT MATERIAL BACKFILL (CLASS V OR C ORDANCE WITH STANDARD SPECIFICATIO	LASS VI)SHALL BE IN NS SECTION 1016.		<b>*</b> B1 58	*5	STR	11'-2"	676
ECT MATERIAL BACKFILL IS TO BE CON	TINUOUS ALONG FILL FACE OF	ŀ	B2 58	<b>*</b> 6	SIR	11'-8″	1016
KWALL FROM OUTSIDE EDGE TO OUTSIDE	EDGE OF APPROACH SLAB.	E	REINFORCI	NG STEE	L	LBS.	1266
THE 4"Ø DRAINAGE PIPE OUTLET(S), SE	E ROADWAY STANDARD DRAWING	GS.	* EPOXY CO	ATED	FFI	I BS	926
A BETWEEN THE WINGWALL AND APPROAC IN THE WATER AWAY FROM THE FILL FA	H SLAB SHALL BE GRADED TO CE OF THE BRIDGE AND SHALL	E	• (111) 01(0	,110 51		203,	524
PAVED. SEE ROADWAY PLANS.			CLASS AA (	CONCRET	E	C.Y.	15.9
ROACH SLAB GROOVING IS NOT REQUIRE	D.		APPR	)ACH	SLAE	B AT EI	3 #2
		ŀ	BAR ΝΟ. * Δ1 13	SIZE #4	TYPE STR	LENGTH 28'-10"	WEIGHT
			A2 13	#4	STR	28'-10"	25Q
SPLICE LENGTHS		- F	• •	•	стр	•	6.76
BAR EPOXY		F	B2 58	*,5 #,6	STR	11'-8"	1016
$\frac{4}{2} - \frac{2}{0} + \frac{9}{1} - \frac{9}{2}$		H	REINFORCIN * FPOXY CO	NG STEE	L	LBS.	1266
<b>*</b> 5 2'-6" 2'-2"			REINFORC	ING ST	EEL	LBS.	926
#6 3'-10" 2'-7"		ŀ		CONCRET	F	C. Y.	15.9
						0.1.	13:4
ARTH ITCH LOCK ROACH -AB -AB -AB -AB -AB -AB -AB -AB	FUTURE SHOULDER T I2" MIN. I2"	OE OF FI	A'-O" MIN.	NE CONTROL R-R ON RESI OVER EART	STANT PIPE TH DIT	CH BLOCK	
TEMPORARY BER	M AND SLOPE [	DRAI	N DET	AIL	<u>S</u>	-	
	PF  S1	ROJEC	T NO. MAC( DN: 1	<u>17</u> 2N 3+C	<u>3P.</u> 9.0	14.R. cou )0 -l	115 jnty 
		DEPA	stati RTMENT	E OF NORTH OF T RALEIG	H CAROLIN RANS SH	PORTAT	ION
	DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	l FOI	BRIDGE R PRES CORE (SUB-R	APP TRES D SL EGIC 90°	ROAC SED AB I NAL SKEW	CONCR UNIT TIER)	B ETE
WSP USA Inc. 434 FAYETTEVILLE STREET	MGINEER HARTIN		RFVTS	IONS		<u> </u>	SHEET NO.
SUITE 1500 RALEIGH, NC 27601 TEL 1 010 836 4040	DocuSigned by M. NO.	BY:	DATE:	NO. BY	:	DATE:	S-17
1 LL: 1.919.830.4040 1 LCENSE NO E 0145	F9EBC057AC1A4EF			3			TOTAL SHEETS
I LIVEINSE INV. F-VIOS	3/16/2021 <b>2</b>			4			17

![](_page_17_Figure_4.jpeg)

![](_page_17_Picture_6.jpeg)

STD. NO. BAS\_30\_90S

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### DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS.PER SO.IN.
- AASHTO M270 GRADE 50W	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT. (MINIMUM)

### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS. ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 ``STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

### CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES. ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

## CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED  $\frac{3}{4}$ " WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS: CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

### DOWELS:

+

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

# STANDARD NOTES

## ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

### **REINFORCING STEEL:**

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES. THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

# STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE  $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE ¾ ″Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - ⅛ ″Ø STUDS FOR 4 - ¾ ″Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF ⅛ ″Ø STUDS ALONG THE BEAM AS SHOWN FOR  $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 -  $\frac{7}{8}$ " Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE. THE CONTRACTOR MAY. AT HIS OPTION. SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST  $\frac{1}{16}$  IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY VIGINCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB. UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

### HANDRAILS AND POSTS:

![](_page_18_Picture_34.jpeg)

STD. NO. SN